- DETAIL, FABRICATE AND ERECT STRUCTURAL STEEL IN ACCORDANCE WITH THE LATEST AISC AND OTHER RELATED CODES, STANDARDS AND SPECIFICATIONS LISTED IN THE PROJECT SPECIFICATIONS, EXCEPT AS MODIFIED THEREIN OR ON THE DRAWINGS.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL MISCELLANEOUS/ORNAMENTAL STEEL NOT SHOWN ON THE STRUCTURAL STRUCTURAL STEEL:
- A. ASTM A992 Fy = 50 KSI FOR ROLLED STEEL WIDE FLANGE SHAPES
- B. ASTM A36 Fy = 36 KSI FOR CHANNELS, ANGLES, PLATES, BARS, RODS,
- C. ASTM A500 GRADE C FOR HSS TUBING Fy = 50 KSI FOR RECTANGULAR 4. HIGH STRENGTH BOLTS: ASTM A325 OR A490, 3/4" DIAMETER MINIMUM UNO
- 5. ANCHOR RODS: ASTM F1554, GRADE 36 UNO WORK STRUCTURAL DRAWINGS WITH ARCHITECTURAL, HVAC, PLUMBING,
- FIRE PROTECTION & ELECTRICAL DRAWINGS FOR CLEARANCES, ATTACHMENTS, ETC.
- 7. ALL FABRICATION AND ERECTION WORK SHALL BE PERFORMED BY AISC CERTIFIED FABRICATORS AND ERECTORS.
- WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY, AWS D1.1 AND SHALL BE PERFORMED BY CERTIFIED WELDERS IN ACCORDANCE WITH THE

AMERICAN WELDING SOCIETY STANDARDS. PROVIDE MINIMUM 1/4" FILLET

- PROVIDE ANGLE WALL ANCHORS, PER PART 4, AISC MANUAL OF STEEL CONSTRUCTION, FOR BEAMS BEARING ON MASONRY WALLS. ANGLE
- ANCHORS SHALL BE WELDED TO BEAMS. 10. CONNECTIONS: WELD OR BOLT CONNECTIONS, AS INDICATED:
- A. CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL CONFORM TO THE REQUIREMENTS OF THE CITED AISC SPECIFICATION.
- B. WHERE THE REACTION VALUES OF BEAMS ARE NOT SHOWN ON THE DRAWINGS, EACH END CONNECTION SHALL BE DESIGNED TO SUPPORT 55% OF THE TOTAL UNIFORM LOAD CAPACITY DERIVED FROM THE ASD VALUE OF THE TABLES AND FORMULA OF THE MAXIMUM TOTAL UNIFORM LOAD IN PART 3, FOURTEENTH EDITION, OF THE AISC MANUAL OF STEEL CONSTRUCTION FOR THE GIVEN MEMBER SIZE, SPAN, AND YIELD STRENGTH. COMPOSITE BEAM CONNECTIONS MUST DEVELOP 75% OF THE TOTAL BEAM ALLOWABLE UNIFORM LOAD CAPACITY, AS GIVEN IN THE AISC TABLES BASED ON SIZE, SPAN, & YIELD STRENGTH.
- THE MINIMUM LENGTH OF CONNECTION ANGLES SHALL BE EQUAL TO ONE HALF THE DEPTH OF THE MEMBER TO BE SUPPORTED.
- D. ONE SIDED CONNECTIONS WILL NOT BE PERMITTED UNLESS SPECIFICALLY DETAILED ON THE DRAWINGS OR SEALED DESIGN CALCULATIONS ARE SUBMITTED WITH THE SHOP DRAWINGS.
- E. THE MINIMUM NUMBER OF BOLTS IN BOLTED CONNECTIONS SHALL BE TWO (2) BOLTS.
- F. MINIMUM 1/4" FILLET WELD SHALL APPLY UNLESS NOTED OTHERWISE. G. MINIMUM SIZE OF CLIP ANGLE SHALL BE L3x3x5/16" UNLESS NOTED
- 11. UTILIZE THROUGH PLATES FOR ALL CONNECTIONS TO TUBES AND PIPE UNLESS SHOWN OTHERWISE.
- 12. TRUSS AND BRACING MEMBER CONNECTIONS SHALL BE DESIGNED FOR THE FORCES INDICATED ON THE DRAWINGS.
- 13. TYPICAL CONNECTION DETAILS INDICATED ON THE STRUCTURAL DESIGN DRAWINGS SHALL DICTATE THE FORM AND GEOMETRY OF THE CONNECTIONS. THE FABRICATOR SHALL DETERMINE OR VERIFY TYPE, SIZE AND NUMBER OF BOLTS, PLATE THICKNESS AND SIZES, WELD SIZES AND LENGTHS, AND ALL REQUIRED INFORMATION NOT SPECIFIED ON THE TYPICAL CONNECTION DETAILS.
- 14. THE DESIGN OF ALL STEEL CONNECTIONS (EXCEPT PREDESIGNED CONNECTIONS THAT HAVE BEEN ENGINEERED ON THESE DRAWINGS) SHALL BE PERFORMED UNDER THE DIRECT SUPERVISION OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT, EMPLOYED BY THE FABRICATOR. THE FABRICATOR'S REGISTERED PROFESSIONAL ENGINEER SHALL SUBMIT COMPLETE DESIGN CALCULATIONS FOR EACH CONNECTION. SUCH CALCULATIONS SHALL SHOW DETAILS OF THE ASSEMBLED JOINT WITH ALL BOLTS AND WELDS
- 15. ALL DESIGN CALCULATIONS SHALL BE SEALED BY THE FABRICATOR'S PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. SHOP DRAWINGS SUBMITTED WITHOUT COMPLETE DESIGN CALCULATIONS WILL NOT BE REVIEWED.
- 16. WELDING ELECTRODES SHALL BE E 70XX OR BETTER. FOR WELDING SYMBOLS WITH NO LENGTH DIMENSION GIVEN, THE WELDING SHALL BE CONTINUOUS BETWEEN ABRUPT CHANGES IN DIRECTION.
- 17. UTILIZE SLIP CRITICAL BOLTS AT ALL MOMENT CONNECTIONS, HANGING CONNECTIONS, BRACING CONNECTIONS, AND COLUMN SPLICES.
- 18. ALL STRUCTURAL STEEL MEMBERS EXPOSED TO THE EXTERIOR SHALL BE GALVANIZED UNLESS NOTED OTHERWISE. THIS INCLUDES BUT IS NOT LIMITED TO MASONRY LINTELS AND SHELF ANGLES, INCLUDING BEARING PLATES AND ANCHOR BOLTS, AND ANY OTHER ITEM LISTED ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS. UNLESS NOTED OTHERWISE, ALL PIPE AND TUBE COLUMNS SHALL BE SEAL WELDED WITH CLOSURE PLATES TO BE AIR TIGHT. ARCHITECTURAL PIPES AND TUBULAR BEAMS SHALL BE PROVIDED WITH 3/8" DIAMETER WEEP
- 19. LOCATION OF ANCHOR RODS SHALL BE CONFIRMED BY A LICENSED SURVEYOR BEFORE ERECTION OF STEEL.
- 20. COLUMNS AND BEAMS WITH BASE, CAP OR END PLATES SHALL HAVE SQUARE CUT OR MILLED ENDS.
- 21. THE FRAMING SHALL BE ERECTED TRUE AND PLUMB. TEMPORARY BRACING SHALL BE PROVIDED AND SHALL REMAIN IN PLACE UNTIL THE LATERAL BRACING SYSTEM IS IN PLACE AND CONNECTIONS OF ALL MEMBERS ARE FINAL AND ALL DECK IS COMPLETELY ERECTED, WELDED AND SCREWED IN
- 22. NON-METALLIC, NON-SHRINK, NON-STAINING GROUT UNDER ALL COLUMN BASE PLATES AND BEAM BEARING PLATES SHALL CONSIST OF A PREMIXED PRODUCT COMPLYING WITH ALL REQUIREMENTS OF CRD-C621, ASTM C827,
- 23. HEADED SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108, TYPE B, GRADE 1010 THROUGH 1020.
- 24. STUD TYPE EXPANSION ANCHORS SHALL BE CARBON STEEL (UNLESS NOTED OTHERWISE ON DRAWINGS) CONFORMING TO THE REQUIREMENTS OF THE MANUFACTURER'S RECOMMENDATIONS. SEE DRAWINGS FOR LOCATIONS AND TYPE.
- 25. ALL STRUCTURAL STEEL MEMBERS (BEAMS AND COLUMNS) ADJACENT TO OR BUILT INTO MASONRY CONSTRUCTION SHALL BE PROVIDED WITH 12 GAUGE GALVANIZED WELD-ON CHANNEL SLOTS AND 3/16" x 1 1/4" HOOKED GALVANIZED ANCHORS, SPACED 16" ON CENTER VERTICALLY AND 24" ON CENTER HORIZONTALLY, MAXIMUM.
- 26. ALL DISSIMILAR METALS TO BE SEPARATED BY ELECTROLYTIC SEPARATORS.
- 27. DO NOT PAINT:
- A. SURFACES OF CONNECTIONS INDICATED AS SLIP CRITICAL.
- B. SURFACES OF CONNECTIONS TO BE FIELD WELDED.
- D. MEMBERS TO BE EMBEDDED IN CONCRETE OR MASONRY.

C. SURFACES TO RECEIVE HEADED SHEAR CONNECTIONS.

E. SURFACES TO RECEIVE SPRAYED ON INSULATION. F. MEMBERS TO BE GALVANIZED.

COLD FORMED LIGHT GAGE STEEL FRAMING:

- ALL COLD FORMED FRAMING SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST AMERICAN IRON AND STEEL INSTITUTE (AISI) "SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
- 3. COLD FORM STEEL MEMBER 16 GA. OR THICKER SHALL BE FORMED FROM 50 KSI YIELD STRENGTH STEEL. COLD FORMED MEMBERS THINNER THAN 16
- GA. SHALL BE FORMED FROM 33 KSI YIELD STRENGTH STEEL. 4. SCREW OF WELD ALL CONNECTIONS. TOUCH UP ALL WELDS WITH ASTM A924 ZINC RICH PAINT.
- 5. AXIALLY LOADED STUDS SHALL BE INSTALLED SO THE ENDS POSITIONED AGAINST THE INSIDE OF THE RUNNER TRACK WEB PRIOR TO FASTENING AND SHALL BE ATTACHED TO BOTH FLANGES OF THE UPPER AND LOWER RUNNER TRACKS.
- 6. DIAGONAL STRAP BRACING SHALL BE CONNECTED TO ALL CROSSING VERTICAL MEMBERS.
- 7. SUGGESTED MEMBER SIZES INDICATED ON DRAWINGS ARE BASED ON DIETRICH INDUSTRIES 1 5/8" FLANGE CSJ SECTION PROPERTIES. ALTERNATE MANUFACTURERS ARE PERMITTED PROVIDED THEY MATCH OR EXCEED SPANNING DIETRICH SECTION PROPERTIES AND MEMBER
- 8. THE CONTRACTOR SHALL SUBMIT CALCULATIONS, CONNECTIONS, AND SHOP DRAWINGS STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED.

- 2. ALL COLD FORMED STEEL FRAMING SHALL BE GALVANIZED. CONFORMING
 - STRUCTURAL DEFERRED SUBMITTALS ARE COMPLETE PACKAGE TO BE SUBMITTED FOR REVIEW THAT INCLUDE DRAWINGS AND CALCULATIONS FOR ALL DELEGATED DESIGN ITEMS AND THIER CONNECTIONS. DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN
 - OEC WILL REVIEW STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN CRITIERIA IS COMPLIANT WITH THE APPROVED CONSTRUCTION
 - 4. STRUCTURAL DELEGATED DESIGN COMPONETS SHALL NOT BE INSTALLED UNTIL APPROVED BY THE BUILDING OFFICAL.
 - 5. STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING SUBMITTALS INCLUDE, BUT ARE NOT LIMITED TO:
 - RELATED COMPONENTS.
 - C. STUCTRUAL STEEL BRACES, GUSSET PLATES, AND CONNECTIONS.
 - NON-STRUCTURAL DELEGATED DESIGN AND SUBSEQUENT DEFERRED SUBMITTALS ARE FOR ITEMS NOT INCLUDED IN THE STRUCTURAL DELEGATED DESIGN SECTION. THESE ARE ITEMS THAT ARE NOT CRITICAL TO THE OVERALL PERFORMANCE OF THE STRUCTURAL SYSTEM BUT THAT

 - 3. OEC WILL REVIEW NON-STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN CRITIERIA IS COMPLIANT WITH THE APPROVED CONSTRUCTION DOCUMENTS.
 - FOR THE DESIGN.
 - ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL SUBMIT A DETAILED GRAFHICAL REPRESENTATION OF THOSE DESIGN LOADS, INCLUDING MAGNITUDE, AND LOACTION. THE GRAPHIC SHALL BE ACCOMPANIED BY DOCUMENTATION INDICATING THAT THE NON-STRUCTURAL ELEMENT DESIGN COMPLIES WITH THE LOADING CRITERIA OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN.
 - NON-STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING SUBMITTALS INCLUDE, BUT ARE NOT LIMITED TO:

- SUPPORT ROOF TOP MECHANICAL EQUIPMENT ON STEEL FRAMES (UNO). MECHANICAL CONTRACTOR MUST SUPPLY SIZES, LOCATIONS AND OPENING REQUIREMENTS TO THE STEEL FABRICATOR PRIOR TO FABRICATION. DEVIATIONS AND/OR MODIFICATIONS TO THE SUPPORTING STRUCTURE AFTER FABRICATION AND/OR ERECTION HAVE COMMENCED NO ADDITIONAL COST TO THE STEEL FABRICATOR, OWNER OR CONSTRUCTION MANAGER.
- SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER, PRIOR TO
- DISTRIBUTED SO AS NOT TO EXCEED THE UNIFORM LOADS INDICATED ON THE DRAWINGS. SUPPORT PIPING FROM ROLLED BEAMS OR CHANNELS FROM ANY LOCATION FROM EITHER TOP OR BOTTOM FLANGES. SUPPORT ITEMS FROM OPEN WEB JOISTS OR WF MEMBERS FROM THE TOP CHORD WHENEVER POSSIBLE. SUPPORT FROM TOP OR BOTTOM CHORDS OF JOISTS SHALL ONLY BE PERMITTED WITHIN 6" OF A PANEL POINT. RESULTING SINGLE HANGING LOAD ON A JOIST SHALL NOT EXCEED 300 POUNDS FOR A K-SERIES JOIST. MORE THAN TWO 300 POUND POINT UNIFORMLY ACROSS STRUCTURAL MEMBERS. HANGERS FROM METAL ROOF DECK ARE STRICTLY PROHIBITED. DEVIATIONS FROM THE ABOVE CRITERIA SHALL BE REMEDIED BY THE INSTALLING CONTRACTOR.
- SUSPENDED FROM PRECAST CONCRETE PLANKS WITH PLANK MANUFACTURER'S REQUIREMENTS
- FROM TRAPEZE HANGERS SHALL BE AS PREVIOUSLY NOTED FOR PIPING AND/OR CONDUIT GREATER THAN 3" IN DIAMETER.
- THE LOADS UNIFORMLY ACROSS STRUCTURAL MEMBERS. WHERE 6" OR GREATER DIAMETER PIPES ARE PERPENDICULAR TO THE
- CENTER THE PIPE AND HANG THE PIPE FROM TWO JOISTS.
- WHERE SUPPORT FROM JOISTS AT LOCATIONS OTHER THAN AT A PANEL POINT IS NOT POSSIBLE, ADDITIONAL WEB REINFORCING IS REQUIRED. PLANS. IT IS THE RESPONSIBILITY OF THE INSTALLING CONTRACTOR TO
- 11. CONTRACTORS INSTALLING MEP & FP SYSTEMS SHALL COORDINATE THE STRUCTURE UNIFORMLY. DO NOT HANG ALL SYSTEMS FROM THE SAME FRAMING MEMBER.
- 12. ALL HANGERS, WIRES, RODS ETC. FOR SUSPENDED ITEMS SUCH AS PIPING, CONDUIT, DUCT WORK, FIRE PROTECTION, SUSPENDED CEILINGS, TECHNOLOGY, ETC. SHALL BE INSTALLED FROM MAIN STRUCTURAL MEMBERS. HANGERS ATTACHED TO METAL ROOF DECK. JOIST BRIDGING OR FROM OTHER NON-STRUCTURAL SYSTEMS IS STRICTLY PROHIBITED.

- THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION AND TESTING PER OBC SECTION 1704.
- 3. THE FREQUENCY OF INSPECTIONS AND TESTING SHALL BE AS OUTLINED IN
- B. SUMMARY REPORTS SHALL BE DISTRIBUTED WEEKLY TO THE OWNER,

STRUCTURAL DELEGRATED DESIGN AND DEFERRED SUBMITTALS:

- SUBMITTALS ARE FOR ELEMENTS, PARTS, OR PORTIONS OF THE OVERALL STRUCTURAL SYSTEM THAT ARE INDICATED OR REFERRED TO ON THESE DRAWINGS AND THAT ARE CRITICAL TO THE PERFORMANCE OT THE OVERALL STRUCTURAL SYSTEM DESIGN CRITERIA HAS BEEN PROVIDED FOR THESE ITEMS IN THE STRUCTURAL NOTES, PLANS, AND DETAILS.

- IMPACT LOADS AND FORCES TO THE STRUCTURAL SYSTEM.
- NON-STRUCTURAL DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND
- INDICATING THAT THE NON-STRUCTURAL ELEMENTS COMPLY WITH THE LOADING CRITERIA PROVIDED HEREIN. SUCH DOCUMENTATION SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE
- ELEMENT WILL IMPACT FORCES IN EXCESS OF LOADS THAT ARE INDICATED PROVIDED HEREIN. THE LETTER SHALL BEAR THE STAMP AND SIGNATURE
- B. STRUCTURAL STEEL STAIRS.

MECH'L, ELEC'L, PLUMBING, FIRE PROTECTION & OTHER SUSPENDED ITEM:

- SUPPORT OF CONDUIT, SPRINKLER, HVAC OR PLUMBING PIPING SHALL BE
- CONNECTIONS TO SUPPORTING STRUCTURAL MEMBERS, WHETHER NEW DAMAGE OR DEFORM THE STRUCTURAL ELEMENTS. WELDING TO OR DRILLING HOLES IN STRUCTURAL MEMBERS IS NOT PERMITTED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. IT IS THE RESPONSIBILITY OF THE INSTALLING CONTRACTOR TO LOCATE AND DISTRIBUTE HANGING LOADS AS REQUIRED SO AS TO NOT EXCEED THE LOAD CARRYING CAPACITY OF THE MEMBER.
- SUPPORTS FROM JOIST ELEMENTS. TRAPEZING IS NOT PERMITTED FOR
- HANGERS AND SUPPORTS FROM THE STRUCTURE SO AS TO DISTRIBUTE
- FEET OF THE END OF THE JOIST. STAGGER HANGERS TO DISTRIBUTE THE LOAD UNIFORMLY ACROSS THE STRUCTURE.
- ROUTING PRIOR TO INSTALLATION SO AS TO DISTRIBUTE THE LOADING TO

- THE OBC TABLE ITEMS LISTED BELOW.
- 4. SEE THE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR SPECIAL INSPECTION AND TESTING.

- STRUCTURAL DELEGATED DESIGN AND SUBSEQUENT DEFERRED
- PROFESSIONAL RESPONSIBLE FOR THIER DESIGN.

- A. OPEN WEB JOINST & GIRDERS, BRACING, CONNECTIONS, AND
- B. PRE-CAST CONCRETE ELEMENTS AND THEIR CONNECTIONS.

NON-STRUCTURAL DELEGRATED DESIGN AND DEFERRED SUBMITTALS:

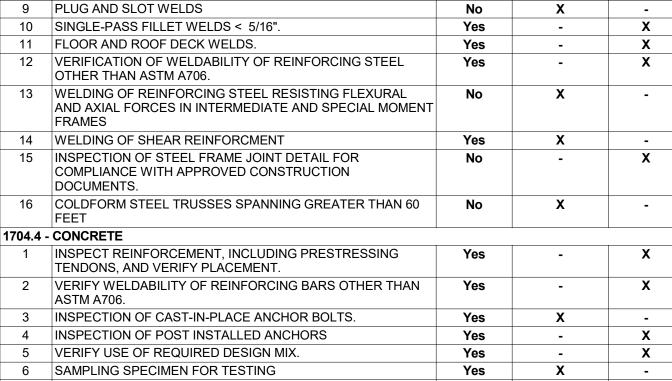
- SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE
- 4. IF THE STRUCTURAL DRAWINGS INCLUDE LOADS TO ACCOMMODATE NON-STRUCTURAL ELEMENTS, THE CONTRACTOR SHALL SUBMIT DOCUMENTS
- WHEN THE NON-STRUCTURAL DEFERRED SUBMITTAL INDICATES THAT THE
- COLD FORMRD STEEL STUDS / JOISTS / HEADERS / JAMBS / TRUSSES.

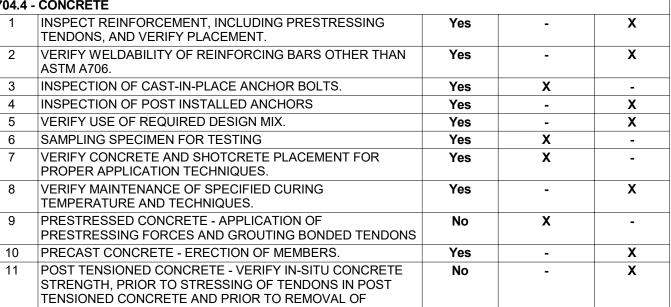
- SHALL BE THE RESPONSIBILITY OF THE SPECIFIED EQUIPMENT SUPPLIER AT
- THE STRUCTURE IN THE VICINITY OF LOADS IN EXCESS OF 400 POUNDS
- LOADS ON A K-SERIES JOIST IS STRICTLY PROHIBITED. STAGGER HANGERS AND SUPPORTS FROM THE STRUCTURE SO AS TO DISTRIBUTE THE LOADS
- OR EXISTING CONSTRUCTION, SHALL BE CLAMPING DEVICE WHICH DO NOT
- COORDINATE CONNECTIONS AND ATTACHMENTS OF ALL MEP/FP ITEMS
- WHERE 6" OR GREATER DIAMETER PIPE ARE PARALLEL TO THE JOISTS,
- COORDINATE THE LOCATIONS WITH THE STRUCTURAL STEEL FABRICATOR AND/OR ERECTOR PRIOR TO INSTALLATION.

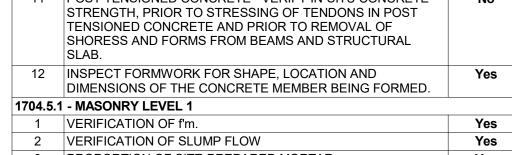
- **SPECIAL INSPECTIONS:**
- 2. THIS WORK SHALL BE PERFORMED BY A SPECIAL INSPECTOR CERTIFIED BY
- PERFORM THE TYPES OF INSPECTIONS AND TESTS SPECIFIED.
- A. DEFICIENCIES SHALL BE REPORTED DAILY TO THE CONTRACTOR.

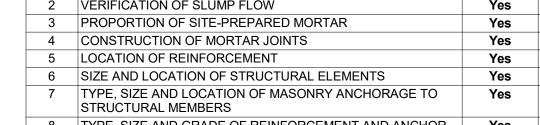
1 MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS 2 INSPECTION OF HIGH-STRENGTH BOLTING - BEARING CONNECTIONS. 3 INSPECTION OF HIGH-STRENGTH BOLTING: - SLIP CRITICAL CONNECTIONS. 4 MATERIAL VERIFICATION OF STRUCTURAL STEEL COLD-FORMED STEEL DECK 5 MATERIAL VERIFICATION OF WELD FILLER MATERIALS. 6 COMPLETE AND PARTIAL JOINT PENETRATION GROOVE 7 MULTI-PASS FILLET WELDS.

8 SINGLE-PASS FILLET WELDS > 5/16".









8 TYPE, SIZE AND GRADE OF REINFORCEMENT AND ANCHOR 9 WELDING OF REINFORCING BARS 10 COLD WEATHER CONSTRUCTION 11 PRIOR TO GROUTING - CLEANING, REINFORCMENT PLACEMENT, GROUT PROPOTION AND MORTAR JOINTS 12 GROUT PLACEMENT

13 PREPARATION OF GROUT AND MORTAR SPECIMEN FOR 1704.5.3 - MASONRY LEVEL 2 1 VERIFICATION OF f'm FOR EVERY 5000 SF VERIFICATION OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR OR GROUT 3 VERIFICATION OF SLUMP

4 PROPORTION OF SITE-PREPARED MORTAR 5 PLACEMENT OF MASONRY UNIT AND CONSTRUCTION OF MORTAR JOINT 6 PLACEMENT OF REINFORCEMENT

7 GROUT SPACE PRIOR TO GROUTING 8 GROUT PLACEMENT 9 SIZE AND LOCATION OF STRUCTURAL ELEMENTS 10 TYPE, SIZE AND LOCATION OF MASONRY ANCHORAGE TO STRUCTUAL MEMBERS

(TRUSSES/COMPOSITE i-JOISTS) CONDUCTED ON THE

1 VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE

ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.

2 VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH

3 PERFORM CLASSIFICATION AND TESTING OF COMPACTED

4 VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT

5 PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT

THICKNESSES DURING PLACEMENT AND COMPACTION OF

SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED

3 INSPECT DRIVING OPERATIONS AND MAINTAIN COMPLETE

PER FOOT OF PENETRATION, DETERMINE REQUIRED

5 FOR STEEL ELEMENTS, PERFORM ADDITIONAL SPECIAL

6 FOR CONCRETE ELEMENTS AND CONCRETE-FILLED

7 FOR SPECIALTY ELEMENTS, PERFORM ADDITIONAL

AND ACCURATE RECORDS FOR EACH ELEMENT.

FOR CONCRETE ELEMENTS, PERFORM TESTS AND

1 INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST

GIRDERS INCLUDING END CONNECTION AND BRIDGING.

PROFESSIONAL IN RESPONSIBLE CHARGE.

1704.9 - CAST-IN-PLACE DEEP FOUNDATION ELEMENTS

CONCRETE OR GROUT VOLUMES.

SECTION 1704.4.

OPEN-WEB STEEL JOIST AND GIRDER

INSPECTIONS IN ACCORDANCE WITH SECTION 1704.3.

ELEMENTS, PERFORM TESTS AND ADDITIONAL SPECIAL

INSPECTIONS AS DETERMINED BY THE REGISTERED DESIGN

ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE),

LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE END-BEARING STRATA CAPACITY. RECORD

ADDITIONAL SPECIAL INSPECTIONS IN ACCORDANCE WITH

1 INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE No

VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM No

INSPECTIONS IN ACCORDANCE WITH SECTION 1704.4.

TYPE AND SIZE OF HAMMER, RECORD NUMBER OF BLOWS

PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP

AND BUTT ELEVATIONS AND DOCUMENT ANY DAMAGE TO

1 VERIFY ELEMENT MATERIALS, SIZES AND LENGTHS COMPLY **No**

2 DETERMINE CAPACITIES OF TEST ELEMENTS AND CONDUCT No

4 VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM No

No

PREMISES OF THE FABRICATORS SHOP.

AND HAVE REACHED PROPER MATERIAL.

1704.8 - DRIVEN DEEP FOUNDATION ELEMENTS

ADDITIONAL LOAD TESTS, AS REQUIRED.

AND ACCURATE RECORDS FOR EACH ELEMENT.

WITH THE REQUIREMENTS.

FOUNDATION ELEMENT.

11 TYPE, SIZE AND GRADE OF REINFORCMENT AND ANCHOR 12 WELDING OF REINFORCING BARS 13 COLD WEATHER CONSTRUCTION 14 PREPARATION OF GROUT AND MORTAR SPECIMENT FOR TESTING 1704.6 - WOOD

HIGH-LOAD DIAPHRAGMS

GREATER THAN 60 FEET

FILL MATERIALS.

COMPACTED FILL.

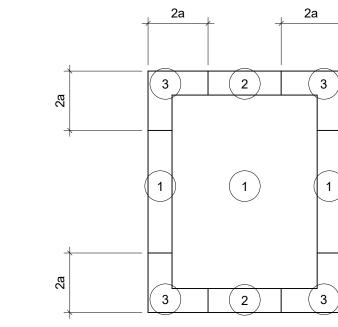
1 FABRICATED LOAD BEARING ASSEMBLIES 3 METAL-PLATE-CONNECTED WOOD TRUSSES SPANNING

TRAPEZING IS PERMITTED FOR MULTIPLE PIPE OR CONDUIT RUNS. LOADS

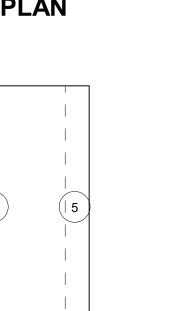
- THE APPROPRIATE INSTALLING CONTRACTOR IS RESPONSIBLE FOR DETERMINING LOADS IMPOSED BY THE INSTALLED ITEMS. STAGGER
- JOISTS, PROVIDE HANGER SUPPORTS FROM EVERY JOIST WITHIN TWELVE
- REFER TO THE JOIST SUPPLIER AND/OR TYPICAL DETAIL INDICATED ON THE

- THE GOVERNING MUNICIPALITY WHERE THE PROJECT IS LOCATED TO
- ARCHITECT, CONTRACTOR, BUILDING OFFICIAL AND STRUCTURAL

SPECIAL INSPECTION REQUIRED CONTINUOUS PERIODIC

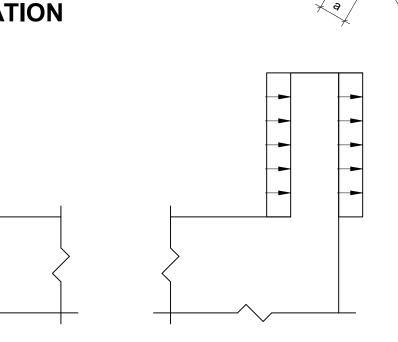


ROOF PLAN



WALL ELEVATION

CASE A: WINDWARD PARAPET



CASE B: LEEWARD PARAPET

			ULTIMATE V	WIND COMP	ONENT AND	CLADDING	LOADS			
COMPONENT	ROOF ZONE 1		ROOF ZONE 2		ROOF ZONE 3		WALL ZONE 4		WALL ZONE 5	
AREA	PRESSURE	SUCTION	PRESSURE	SUCTION	PRESSURE	SUCTION	PRESSURE	SUCTION	PRESSURE	SUCTION
10 SF	16	-24	16	-40	16	-60	22	-24	22	-29
20 SF	16	-23	16	-36	16	-50	21	-23	21	-27
50 SF	16	-22	16	-30	16	-36	20	-21	20	-25
100 SF	16	-22	16	-26	16	-26	19	-20	19	-23

END ZONE WIDTH IN FEET = 10 PERCENT OF BUILDING LEAST HORIZONTAL

HORIZONTAL DIMENSION OR 3 FEET.

z: HEIGHT ABOVE GROUND IN FEET

EAVE ANGLES < 10°

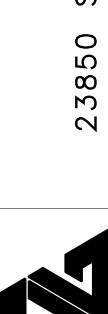
DIMENSION OR 0.4h WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF LEAST

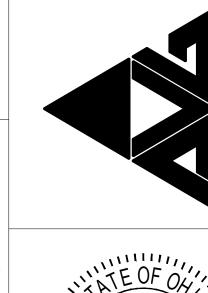
MEAN ROOF HEIGHT IN FEET, EXCEPT THAT EAVE HEIGHT SHALL BE USED FOR

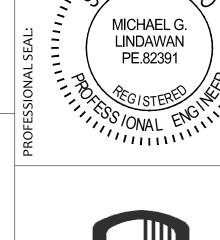
CASE	PARAPET ZONE	EFFECTIVE WIND AREA								
		10 SF	20 SF	50 SF	100 SF	200 SF	500 SF			
CASE A	4	55	50	42	37	36	35			
CASE A	5	75	63	48	37	36	35			
CASE B	4	-38	-36	-34	-32	-30	-27			
CASE B	5	-44	-41	-37	-34	-31	-27			

2203-2

 \mathcal{O}









GENERAL NOTES

PROJECT NO 2203-2 DRAWN BY OEC CHECKED BY OEC

SCALE: 12" = 1'-0"